

1 **ITS APPLICATION: DESIGN AND EVALUATION OF AN INTELLIGENT DILEMMA**
2 **ZONE PROTECTION SYSTEM FOR A HIGH SPEED RURAL INTERSECTION**

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4 **Gang-Len Chang**

5 Ph.D., Professor
6 Department of Civil Engineering
7 1173 G. Martin Hall
8 University of Maryland at College Park
9 20740, MD USA
10 (301) 405-1953, (301) 405-2585, gang@umd.edu

11
12 **Mark L. Franz (Corresponding Author)**

13 MSCE, Ph.D. Candidate
14 University of Maryland
15 Department of Civil & Environmental Engineering
16 1173 Glenn L. Martin Hall
17 College Park, MD 20742
18 mfranz1@umd.edu

19
20 **Yue Liu**

21 Ph.D., Assistant Professor
22 Department of Civil Engineering and Mechanics
23 University of Wisconsin at Milwaukee
24 P.O. Box 784.
25 Milwaukee, WI 53201-0784
26 Tel: 414-229-3857, liu28@uwm.edu

27
28 **Yang (Carl) Lu**

29 Research Assistant
30 University of Maryland
31 Department of Civil & Environmental Engineering
32 1173 Glenn L. Martin Hall
33 College Park, MD 20742
34 yanglu@umd.edu

35
36 **Ruihua Tao, Ph.D.**

37 Maryland State Highway Administration
38 Office of Traffic and Safety
39 7491 Connelly Drive, Hanover, MD 21076
40 rtao@sha.state.md.us

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1 **ABSTRACT**

2 This paper presents the design and evaluation of a dilemma zone protection system that utilizes
3 the dynamic detection technology to track individual vehicles as they approach an intersection of
4 interest. A high-speed rural intersection in Maryland experiencing a high frequency of crashes
5 was selected for system installation and evaluation. Data collected from 3 sensors, designed
6 specifically for tracking individual vehicles, were deployed along the target approach were used
7 in real time to control the signal logic, providing green or all-red extensions when the pre-
8 defined parameters of detected vehicles are met. To evaluate the performance of the system
9 design and the effectiveness of the associated parameters, a field test was further conducted. The
10 data analysis included the identification of falsely-called red extensions (related to efficiency)
11 and missed red extensions (related to safety) to assess the overall performance of the newly
12 installed system. The field observation results indicate that the newly designed dynamic
13 dilemma zone protection system using an all-red extension offers distinct advantages over
14 traditional systems by providing additional protection to high-speed vehicles even when they are
15 in the “cannot go zone” and make an incorrect decision to go.

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1 **1. INTRODUCTION**

2 Improving traffic safety has increasingly been regarded as a priority transportation issue in most
 3 states. A tremendous amount of resources has been invested on improving safety and efficiency
 4 at signalized intersections. Although programs such as driver education, red-light camera
 5 deployment, and operational improvements to roadway geometry have all contributed to a safer
 6 driving environment, significantly reducing traffic signal-related crashes remains a challenging
 7 task.

8 In review of the literature, it is evident that the first intersection dilemma zone model,
 9 also termed “Type-I Dilemma,” was developed by Gazis, Herman, and Maraduin (1) in their
 10 landmark paper, called the GHM Model. The paper defines the dilemma zone as a range in
 11 which a vehicle approaching the intersection during the yellow interval can neither safely clear
 12 the intersection nor stop comfortably at the stop-line (Figure 1). The existing practice for
 13 computing the dilemma zone is based on the following kinematics equation:

14
$$x_{dz} = x_c - x_0 = v_0 \delta_2 + \frac{v_0^2}{2a_2^*} - v_0 \tau + (w + L) - \frac{1}{2} a_1^* (\tau - \delta_1)^2 \quad (1)$$

15 where:

16 x_c = the critical distance for a smooth “stop” under the maximum deceleration rate;

17 x_0 = the critical distance for “intersection clearance” under the maximum acceleration
 18 rate;

19 τ = duration of the yellow interval (sec);

20 δ_1 = reaction time-lag of the driver-vehicle complex (sec);

21 δ_2 = decision-making time of a driver (sec);

22 v_0 = approaching speed of vehicles (ft/sec);

23 a_1 = average vehicle acceleration rate (ft / s²);

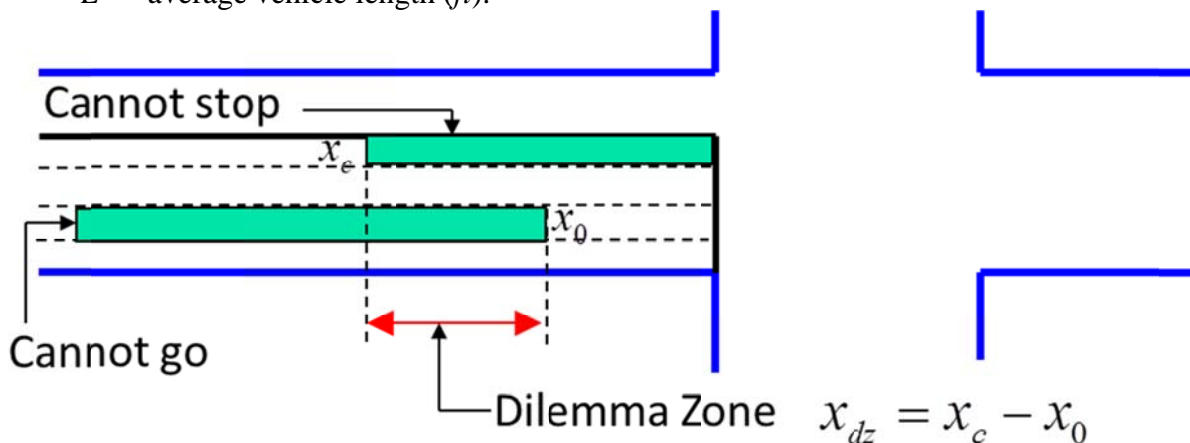
24 a_1^* = maximum acceleration rate of the approaching vehicles (ft / s²);

25 a_2 = average vehicle deceleration rate (ft / s²);

26 a_2^* = maximum deceleration rate of the approaching vehicles (ft / s²);

27 w = intersection width (ft); and

28 L = average vehicle length (ft).



29 **FIGURE 1** Graphical illustration of the dilemma zone at signalized intersections
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1
2 The other dilemma, termed “Type-II Dilemma,” was proposed to accommodate the
3 problem of indecision when both stopping and intersection clearance maneuvers can be
4 executed. The term defines the dilemma zone as the range in which 10 to 90 percent drivers
5 decide to stop (2). Zeeger et al. (3) also proposed a measuring method, termed as “option zone”,
6 in which 90 percent of vehicles will stop and 10 percent will choose to go through the
7 intersection under the condition of stochastic traffic distribution.

8 It is noticeable from Equation-1 that both the length and the location of a dilemma zone
9 may vary with the approaching vehicle speeds, driver reaction times, and vehicle
10 acceleration/deceleration rates. A high-speed intersection is likely to contain several different
11 dilemma zones for different groups of the driving population (e.g., conservative or aggressive).
12 Thus, intersection dilemma zones are more likely to be spatially distributed over a wide range,
13 rather than a constant as computed in existing practices. As such, design of effective counter
14 measures to eliminate the dynamic dilemma zone at high-speed intersections has emerged as an
15 imperative but difficult research issue in the traffic safety community.

16 In Maryland, due to a high frequency of crashes that may be related to dilemma zone
17 scenarios, the Maryland State Highway Administration (MDSHA) installed a state-of-the-art
18 dilemma zone protection system at the intersection of US 40 (Pulaski Highway) and Red Toad
19 Road in North East, MD. This intersection has experienced a total of 89 crashes from 2000-2010,
20 40 of which were right-angle crashes that *may* be corrected by sufficient dilemma zone
21 protection (4). The installed system used specially designed traffic detectors to provide real-time
22 vehicle tracking as vehicles approached the intersection on the major approach. The data was
23 then used in signal timing decisions, including dilemma zone protection.

24 25 **2. LITERATURE REVIEW**

26 Driver responses at signalized intersections have been investigated along with the
27 dilemma zone issue in the literature since its initial study by Gazis et al. (1). They indicated that
28 incompatibility frequently exists between a driver’s desire to comply with the yellow-interval
29 indication and the encountered constraints. Olson and Rothery (5) conducted field observations
30 at five intersections and found that drivers tend to take advantage of a long yellow interval and
31 view it as an extension of the green interval. Their research concluded that driver behavior does
32 not seem to be affected by the yellow-interval duration, especially since most motorists do not
33 even know the typical phase duration. Another type of dilemma associated with a driver’s
34 decision making, termed as “Type-II Dilemma,” was proposed to accommodate the problem of
35 indecision when both stopping and intersection clearance maneuvers can be executed. Zeeger et
36 al. (3) also proposed a method, termed “option zone,” where 90 percent of the vehicles stop and
37 10 percent go under various traffic conditions. Liu et al. (6, 19-20) presented the results of an
38 empirical study on dilemma zones for different driver groups at signalized intersections using a
39 specially designed video-based system. Their empirical results revealed that the dynamic nature
40 of the dilemma zone often varies with the behavior of the driving population; they also
41 concluded that the commonly used practice of extending the yellow phase duration may not be
42 effective.

43 In studying a driver’s response to the yellow-light phase, Van der Horst and Wilink (7)
44 indicated that such a process is governed by a multitude of factors, including driver attitude and
45 emotional states, the crossing ability before the red phase, the consequence of the decisions to
46 stop or go, interactions with other drivers, and the vehicle’s approaching speed. They used

1 extensive numerical analyses to illustrate the complex decision-making process and its relations
2 with associated factors. Their employed parameters were also adopted in later studies by Milazzo
3 et al. (8), Koppa (9), BMI (10), Shultz et al. (11), and the Green Book (12).

4 In classifying driver responses during the yellow interval and identifying potential
5 affecting factors, Shinar and Compton (13) observed more than 2000 drivers over a total of 72
6 hours at six intersections. They concluded that male drivers are more likely than female drivers
7 to take aggressive actions; senior drivers in comparison with young drivers are less likely to
8 manifest aggressive driving patterns during a yellow interval; the presence of passengers was
9 associated with lower rates of aggressive driving; and the likelihood of taking aggressive actions
10 increases with a driver's value of time.

11 More recent studies (14-16) have explored the use of wide area detectors to provide real-
12 time information for signal control, including dilemma zone protection. Each of these studies
13 showed the potential for using dynamic dilemma zone protection to improve the safety and
14 efficiency of a target intersection.

15 **3. DESIGN OF DILEMMA ZONE PROTECTION SYSTEM**

16 Despite the impressive contributions reported in the literature, many critical research
17 issues remain in the design and evaluation of a dynamic dilemma zone protection system. The
18 goal of this research was two-fold; the first task is to design a dynamic actuated signal control
19 system that provides dilemma zone protection with an all-red extension based on the target
20 vehicle's real-time speeds and distances to the stop-bar. Next, the study evaluated the system
21 design using independently collected field data. The details of the study are discussed in the
22 following sections.

23 **The Study Site**

24 Serving as a primary arterial in Cecil County, Maryland, US 40 is a four-lane, median-divided
25 highway with a posted speed limit of 55 mph and isolated intersection control. It has a high
26 traffic speed and long spacing between intersections, and thus is inherently subject to dilemma
27 zone safety concerns. The target intersection at Red Toad Road provides a left turn bay for each
28 approach on US 40 and has a historic pattern of crashes that may be corrected by sufficient
29 dilemma zone protection.

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33 The traffic signal at the US 40 and Red Toad Road intersection is controlled by a semi-
34 actuated two-phase system with no pedestrian accommodation. The green interval for US 40 is
35 held unless there is a call from Red Toad Road. The minimum green time for US 40 is 25
36 seconds after a call is received from Red Toad Road. The maximum green time for US 40 is 60
37 seconds (90 seconds in peak periods) with the gap-out logic controlled by sensors. The yellow
38 interval for US 40 is 5.5 seconds and a fixed all red interval of 3 seconds is incorporated.
39 Dilemma zone protection is provided by extending the all-red interval by up to an additional 2.5
40 seconds for vehicles meeting predefined thresholds during the default all-red interval for US 40.
41 These thresholds are based on the detected vehicles' speeds and distances to the intersection stop
42 line (details in system design subsection), this all-red extension may be called even if the green
43 duration has not been extended to its maximum.

1 **Pre-Design Survey and Analysis**

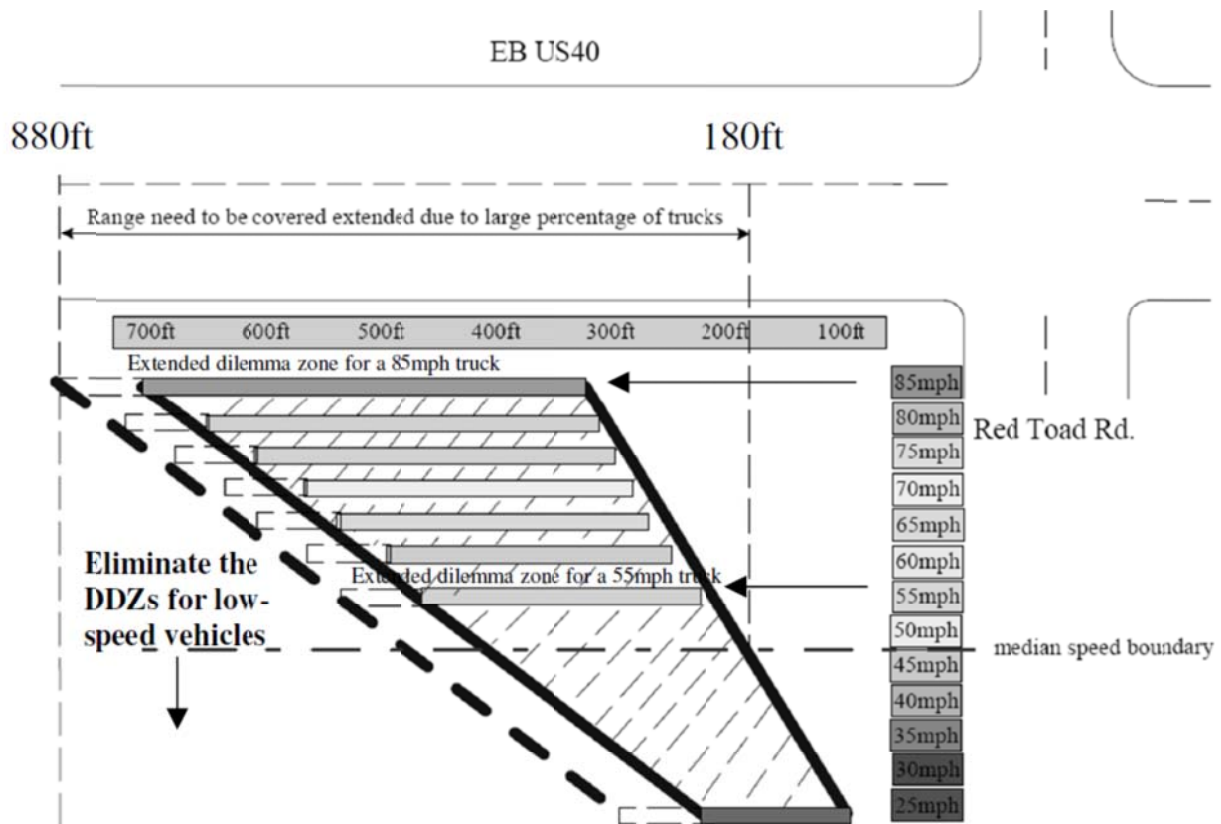
2 To understand the flow characteristics of the target intersection, a pre-design survey was
 3 conducted using video recording devices. The survey covered both the eastbound and westbound
 4 approaches of US 40 and collected space mean speed, vehicle classification and individual
 5 vehicle response (stop or go) to the yellow interval. Table 1 summarizes the key findings from
 6 this pre-design survey.

7

TABLE 1 Summary of pre-design survey findings

Parameter	Westbound	Eastbound
Mean Speed (mph)	49.2	49.6
Median Speed (mph)	49.9	50.4
Std. Deviation (mph)	12.3	11.7
Minimum Speed (mph)	19.6	21.6
Maximum Speed (mph)	86.7	79.3
85 Percentile Speed (mph)	62.4	62.0

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EB Results, (WB has similar patterns)

9
 10 **FIGURE 2** The range of the normal and extended dilemma zone due to the large percentage of
 11 trucks

12 **System Design**

13 Figure 2 illustrates the spatial distribution of dilemma zones for vehicle groups
 14 approaching the intersection at different speeds. Vehicles traveling below 50 mph were observed
 15 to be more likely to stop during the yellow-phase. In addition, most vehicles involved in
 16

1 incidents were approaching the intersection at the speeds of 55 mph or above. Thus, 55 mph was
2 set as the threshold speed for the dilemma zone monitoring and protection system. (The key
3 system design issue was how to monitor those vehicles trapped within the dilemma zone. To
4 ensure each trapped vehicle can safely clear or stop at the intersection, the proposed system
5 should also have the capability to track each vehicle's speed and distance to the intersection stop
6 line.

7 The extended all-red interval is designed to provide extra time for those vehicles trapped
8 in the dilemma zone, especially for high-speeding vehicles in the far end of the protection zone,
9 to safely clear the intersection. The methodology of "looking" for vehicles at the onset of the
10 default all-red period allows for an all-red extension even if the maximum green time has not
11 been achieved. Additionally, this logic will provide protection for those motorists who make the
12 incorrect decision in attempting to clear the intersection when they are in fact in the "cannot go
13 zone", upstream of the dilemma zone, described in [Figure 1](#).

14 Conceivably, the key to the success of such a system is to identify an effective traffic
15 sensor system that can reliably monitor the speed and location of each vehicle within the target
16 zone of 880ft. Since most traffic sensors for urban traffic control are designed for point
17 measurement (i.e., either loop-based or narrow-beam radar detectors), the proposed protection
18 system must rely on either a series of point sensors or a wide-beam radar or microwave sensor.

19 A review of the available traffic sensors in the market for this study showed that
20 Wavetronix® has developed a microwave detector (the SmartSensor Advance) to address the
21 limitations of traditional dilemma zone protection with loop detectors (18). Such a specially
22 designed sensor functions like a series of loop detectors and can dynamically track vehicles as
23 they approach the intersection. In fact, vehicle speeds and distances are updated every 0.1
24 seconds. The implemented sensor has a detection range of 500ft within which the sensor can
25 continuously measure vehicle speeds and distance from the intersection stop bar. The computing
26 module within the sensor continuously updates the estimated time-varying arrival times of each
27 detected vehicle and informs the signal controller to take proper action. Using a time-based
28 rather than a distance-based tracking method, the dilemma zone protection system can ensure a
29 safe intersection clearance or stop of each vehicle based on its speed evolution within the
30 detection zone.

31 The detection range of a sensor is only 500ft, which is shorter than the EB protection
32 zone of 880ft at the US 40 and Red Toad Road intersection. The proposed system design uses a
33 seamless combination of two such sensors to provide ample dilemma zone protection for the EB
34 approaching vehicles ([Figure 3](#)). The first sensor was placed on the signal mast and the second
35 signal was placed in 375 ft from the stop bar, covering upto 875 ft. This design decision is based
36 on severity of the crash history for the EB direction. Because the intersection is operated under
37 an actuated control, one identical sensor was also deployed in the WB (covering out to 500 ft
38 from the WB stop-bar) to ensure the proper function of the controller when called by the sensors
39 to take preset strategies.

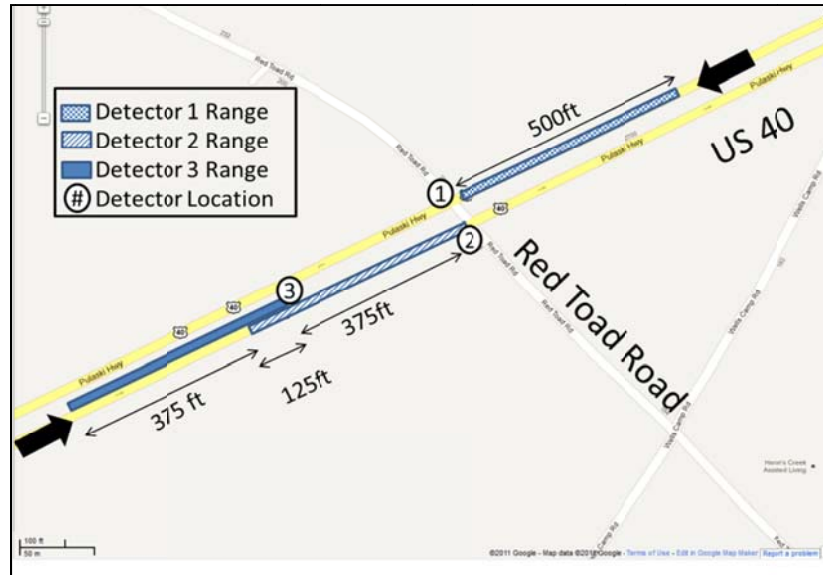


FIGURE 3 Sensor locations and detection range for the final design (17)

System Logic

The logic for the system to decide to extend the all-red interval was based on estimated time of clearance of the intersection. The time of clearance was determined by speed and distance of vehicles detected within 3 seconds of the onset of the US 40 red interval. The time of clearance is updated every 0.1 seconds until the end of the default all-red interval. At this point, the final decision on all-red extension is determined. In the proposed design, the time of clearance was used to control the signal logic.

Specifically, the system determines the time for a target vehicle to clear the intersection at the onset of the US 40 red interval, updating that decision every 0.1 seconds. If the time of intersection clearance is less than the remaining default all-red interval, then no all-red extension is called. On the other hand, if the time of intersection clearance is greater than the remaining default all-red interval but less than or equal to the remaining default all-red interval plus 2.5 seconds (the maximum all-red extension), an all red-extension is called. The length of the all red-extension is a function of the detected speed and distance at the end of the default all-red interval. In the instance when the time of intersection clearance is greater than the remaining default all-red interval plus 2.5 seconds (the maximum all-red extension), the system assumes the vehicle will stop and no all-red extension is provided.

The following operational features for the final design of the dilemma zone protection system were based on conservative pre-design survey observations:

- Call a green extension after reaching the minimum green time if a vehicle was detected within 500ft of either intersection stop bar with a minimum speed of 27 mph;
- Call an all-red extension if a vehicle is detected within 500ft of either US 40 approach at a minimum speed of 56 mph at the onset of the US 40 red interval. The length of the extended all-red interval is determined by the vehicle's speed and its distance from the stop bar with a maximum extension of 2.5 seconds.
- Additional dilemma zone protection for EB US 40 was provided by sensor 3. The section of EB US 40 covered uniquely by the second EB sensor (from 500ft to

1 875ft relative to the EB stop bar) was used only for all red extension. Within this
2 range, a vehicle must be detected with a minimum speed of 67 mph for an all red
3 extension to be called.

4 Thus, vehicles detected under these threshold speeds are assumed to stop, which was
5 consistent with field observations after system deployment. In contrast, those vehicles detected at
6 or above the threshold speed will activate the all-red extension function. The length of the all red
7 extension is based on time of intersection clearance, a function of detected distance and speed at
8 the end of the end of the US 40 default all-red interval. Note that those control parameters are
9 subjected to change if periodical field observations have detected significant changes in driving
10 responses to the target signal with the deployed sensor system.

13 **4. SYSTEM EVALUATION**

15 **System Evaluation Methodology**

16 To evaluate the performance of the installed system, several candidate data collection
17 plans were considered. While a bird's eye video is a convenient method for mimicking the
18 continuous microwave detection system, this site was on a level grade, making this method
19 infeasible. Additionally, the specific nature of the parameters needed to call the all-red extension
20 requires high accuracy measurement of speeds at given distances. Thus, non-perpendicular views
21 of approaching vehicles may introduce parallax-related errors. With these considerations in
22 mind, the research team decided to conduct an in-depth data collection at only the eastbound
23 approach of US 40, using both perpendicular videos and tube detectors.

24 The data collection plan used five video recording cameras and four tube detectors. Four
25 of the video cameras were used to track vehicle speeds at predefined distances from the EB
26 intersection stop bar, by measuring the time to traverse a known perpendicular distance in each
27 video frame.

28 To measure vehicle speeds at each preset distance within the system's detection range,
29 video cameras and tube detectors were alternated every 100ft, starting at 200ft from the EB US
30 40 intersection stop bar. Since a microwave sensor only reaches to 875ft from the intersection,
31 the final tube detector was placed at this location rather than at 900ft from the stop bar. The
32 remaining video camera was used to capture the EB US 40 signal phases and timings. [Figure 4](#)
33 provides a summary of the equipment and deployed locations on EB US 40. To determine when
34 an all-red extension was called and from which approach, the research team used the signal log
35 files provided by the MDSHA signal shop that includes all red-extension events recorded by the
36 intersection's actuated controller.

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Data Reduction

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With data coming from multiple sources with independent internal clocks, the data from each data source was synchronized using handheld GPS units to ensure consistency between all devices. Before starting the data collection, a GPS unit was placed next to each of the tube detectors and the signal clock to estimate the time offset of each data source. The offset between each data source’s internal clock and the GPS clock was calculated and applied to the respective data files. Similarly, each video began by recording the GPS unit to establish a universal time for all video sources. The GPS time was input into each video using video reduction software.

Using the video data from high-precision camcorders, the research team members were able to compute vehicle speeds by measuring their times to traverse a marked distance in each video frame. The distance was marked using construction cones placed on both sides of EB US 40. The time to traverse each marked distance was determined by creating time stamps for each vehicle as it entered and exited the measurement zone. To improve the accuracy of the manual video reduction, this study also produced a specially-designed computer program to clearly mark the entrance and exit of the measurement zone as well to slow the video down to 1/128 play back speed. This software was used to create timestamps for each EB US 40 phase change during the analysis period and also to synchronize the time stamp clock to the recorded GPS clock. A snapshot of this software is provided in [Figure 5](#).



1
2 **FIGURE 5** Snapshot of video reduction software and vehicle identification that activated the all-
3 red extension
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5 The tube detector data was extracted from the devices using the associated “out-of-the-
6 box” software. Each tube detector has its own internal clock to create the time stamps for
7 vehicle detections. Thus, each tube detector had a unique offset relative to the GPS clock. The
8 time offset for each tube detector was computed and applied to each respective data file.
9 Similarly, the time offset for the signal controller was incorporated into the all-red extension log
10 file.

11 **Evaluation Analysis & Results**

12 To evaluate the performance of the dilemma zone protection system, the analysis focused on
13 vehicles detected within the first three seconds of the red interval for EB US 40. During this
14 period, the system looked for vehicles at or above the pre-defined threshold speeds based on their
15 distances from the stop bar. The threshold speed for vehicles within 500ft from the intersection
16 stop bar was 56 mph; for those from 500ft to 875ft the speed was set to be 65 mph. The goal of
17 this analysis was to evaluate the system performance by identifying any incorrect signal calls,
18 including false negatives (at the cost of safety) and false positives (at the cost of efficiency). The
19 analysis procedure to evaluate the system’s detection accuracy consists of the following steps for
20 each vehicle data source (4 tube sources, 4 video sources):

21
22 Step 1: Identify the signal status when there are vehicles detected in the protection zone,
23 using an algorithm developed in this study that can match the time stamps for each
24 detected vehicle to a signal phase.

25 Step 2: For vehicles detected in a red interval, subtract the starting time of the associated
26 red interval from each vehicle’s detection time. If the difference between the start of the
27 red time stamp and the detection time of the target vehicle was less than three seconds,
28 this observation had the potential to call the all-red extension.

1 Step-3: Determine if the extended all-red interval should be activated by comparing the
2 speeds of the vehicles detected within three seconds of red with the threshold speeds
3 mentioned above, based on the distance from the stop bar in which the detection took
4 place. For those events that warranted the all red extension, verify that the extension was
5 indeed called using the signal log file.

6 Step-4: For all red extensions called, verify that there was at least one vehicle with the
7 detection range meeting the threshold criteria for an all red extension.
8

9 During the four-hour observation period, a total of 164 red intervals was observed at US
10 40 with 521 vehicles detected within three seconds of the onset of a red interval, within which
11 495 (95 percent) were passenger vehicles, and just 26 (5 percent) were commercial trucks. Only
12 one all-red extension was called by the EB approach of US 40 during the observation period.
13 This single event provided the only opportunity to check for detection accuracy.

14 Using the time stamp for the red extension from the signal log file, the single red interval
15 containing the all-red extension was identified. To check the validity of the call, vehicles
16 detected within three seconds of the onset of the target red interval were analyzed from all
17 vehicle data sources. Comparing the detected vehicle's speed with the threshold speed at the
18 target distance confirmed that the call was indeed validated where a van traveling at 57.5 mph
19 was observed at the distance of 400ft from the stop bar. This detection called for extending the
20 all-red interval for an additional 1.1 seconds.

21 Note that the image data at 400ft from the stop bar was captured by the camcorder video.
22 Thus, the vehicle that activated the all-red extension was able to be positively identified (see
23 [Figure 5](#)). Using the video to capture the signal phases, it was possible to observe the event in
24 which vehicle ran over the extended red interval just before the side street (Red Toad Road)
25 traffic was released. Screenshots of this event at the end of the three seconds of the default all-
26 red and at the end of the all-red extension are shown in Figures 7 and 8, respectively.
27



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29
30 **FIGURE 6**

31 (A) Vehicle position after 3 seconds of all-red. (B) Vehicle position at end of all-red extension.
32

33 As shown in [Figure 6A](#), three seconds after the start of the default all-red interval, the
34 vehicle that triggered the all-red extension was approximately 150ft from the EB US 40
35 intersection stop bar. [Figure 6B](#) depicts the vehicle's position at the end of the all-red extension,
36 barely clearing the intersection just before the conflicting traffic from Red Toad Road was
37 released.

38 The analysis used the same procedure to investigate false negative calls that come at the
39 cost of safety. Each red-interval in which the all-red extension was not called provided an

1 opportunity to evaluate the possibility of false negatives. Again, the speeds of all vehicles
2 detected within three seconds of the onset of a red interval were compared with the threshold
3 speed at each respective distance for detection. Of the remaining 520 vehicles detected within
4 three seconds of the onset of a red interval in US 40, none met the criteria that required the
5 system to call the all-red extension nor did any of these vehicles run the red signal.

6 In addition to validating the single all red extension, the analysis also checked to see if
7 any vehicles are trapped in the traditional dilemma zone (our systems look for vehicles at the
8 onset of red, rather than at the onset of yellow). Using Equation-1, the size and location of a
9 dilemma zone was calculated with the following parameter values: $\tau=5.5$ sec, $W=70$ ft, $L=12$ ft
10 $\delta_1=1.14$ sec, $\delta_2=1.14$ sec, $a_1^*=11.2$ ft/s², and $a_2^*=16.2$ ft/s².

11
12 Upon testing for the existence of a traditional dilemma zone at 1 mph increments, the
13 analysis discovered that a dilemma zone did not exist unless an approaching vehicle exceeded 76
14 mph beyond the onset of the yellow interval. Interestingly, only a single vehicle exceeding the
15 threshold speed of 76 mph was detected during the yellow interval. This vehicle was detected
16 with a speed of 77 mph which corresponds to 9 foot dilemma zone existing from 692-701 ft from
17 the stop bar. However, the vehicle traveling at 77 mph was detected at a distance of 875ft from
18 the stop bar; well within the “cannot go zone”. Thus, the yellow interval of 5.5 seconds
19 effectively prevented any vehicles from being trapped in a dilemma zone during this study.

20 It is also important to realize that the vehicle that called the all red extension would have
21 not been protected using a traditional dilemma zone protection system which looks for vehicles
22 at the onset of yellow. Assuming the vehicle that called the all-red extension approached the
23 intersection at the same speed (57.5 mph) in which it was detected at 400 ft, the vehicle would
24 have been located at approximately 932.4 ft at the onset of yellow. This distance would not likely
25 be covered by a traditional dilemma zone protection system as such a system would assume this
26 vehicle would comfortably stop before the end of the all-red interval. Thus, a green extension or
27 all red extension (if max green had been achieved) would not have been called. In doing so, the
28 vehicle would have entered the intersection near the termination of the default all red interval of
29 3 seconds (Figure 6B), potentially resulting in a conflict with vehicles entering from the minor
30 road (Red Toad Road). This observation emphasizes a distinct advantage of looking for vehicles
31 at the onset of red, rather than at the onset of yellow. A driver who cannot clear the intersection
32 but makes the incorrect decision and attempts to do so can still be protected by extending the all
33 red interval. Such an instance was clearly captured and indicated in the video during this study.

34 35 **5. CONCLUSIONS AND RECOMMENDATIONS**

36 The goal of this research was to design and evaluate a dynamic dilemma zone protection system
37 at a high-speed rural intersection of US 40 and Red Toad Road, in Northeast Maryland. The
38 designed system took advantage of state-of-the-art vehicle detection technology to control the
39 traffic signal logic. In doing so, the system was able to effectively prevent a potential incident
40 during the brief evaluation period. Perhaps more importantly, the system had no false-negative
41 calls in which a vehicle traveling above the threshold speed at a given distance from the stop-bar
42 was not given an all-red extension to clear the intersection.

43 Based on the research findings, a dynamic dilemma zone protection using an all-red
44 extension offers some distinct advantages over more traditional dilemma zone protection
45 systems. First, the dynamic dilemma zone protection system provides protection based on an
46 estimated time of arrival at the intersection. Traditional loop detectors provide protection only

1 for those vehicles traveling at the design speed and at the location of the detector. Next, the use
2 of the all-red extension allows the system to provide additional protection to high speed vehicles
3 even if the maximum green time has not been achieved. Therefore, if a driver makes the
4 incorrect decision in trying to clear the intersection when in the “cannot go zone”, the system can
5 still provide additional time for clearance.

6 Despite the promising results of this study, more research needs to be conducted to
7 develop a robust dynamic dilemma zone protection system. Such a system should be able to
8 track and predict possible responses to the yellow signal. Based on those responses, the detection
9 system can appropriately control the traffic signal to provide sufficient dilemma zone protection
10 when needed. Future research may combine an automated enforcement system that provides the
11 all-red extension but also tickets the drivers who enter the intersection during the all red-interval.

12

13

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REFERENCES

1. Gazis, D., Herman, R., and Maradudin, A., 1960. "The problem of the amber signal light in traffic flow," *Operations Research*, Vol. 8, No. 1, 112-132.
2. Technical Committee 18, ITE Southern Section. "Small-Area Detection at Intersection Approaches." *Traffic Engineering*, Feb 1974, pp. 8-17.
3. Zegeer, C.V., and R.C. Dean, 1978. "Green-extension systems at high-speed intersections." *ITE Journal*. 48, pp. 19-24.
4. Maryland Accident Analysis Reporting System (MAARS). Crash History for the US 40 at Red Toad Road Intersection from 2000-2010. Access Nov. 11, 2011. Retrieved from: <https://maars.cattlab.umd.edu/#>
5. Olson, P.L. and Rothery R.W., 1961. Driver Response to the Amber Phase of Traffic Signals. *Operations Research*. Vol. 8 No. 5.
6. Liu, Y., Chang, G. L., Tao, R., Hicks, T., and Tabacek, E., "Empirical Observations of Dynamic Dilemma Zones at Signalized Intersections," *Transportation Research Record*, 2035, 122-133. 2007.
7. Van der Horst, R. and A. Wilmlink. "Drivers' Decision-Making at Signalized Intersections: An Optimization of the Yellow Timing." *Traffic Engineering & Control*. Crowthorne, England, December 1986, pp. 615-622.
8. Milazzo, J., J. Hummer, N. Roupail, L. Prothe, and J. McCurry, 2002. The Effective Dilemma Zones on Red Light Running Enforcement Tolerances. Transportation Research Board 81st Annual Meeting, Washington, DC.
9. Koppa, R. Human Factors, Chpt. 3 in Revised Monograph on Traffic Flow Theory, and Update and Expansion on the Transportation Research Board (TRB) Special Report 165, "Traffic Flow Theory," published in 1975, 1992.
10. BMI. Developing a Uniform Policy for Traffic Signal Timing Change Intervals. Report for the Maryland State Highway Administration. 2002
11. Shultz, G. and M. Babinchak. Methodology Study for the Consumer Braking Information Initiative. NHTSA 99.658 3-1, 1998.
12. American Association of State Highway and Transportation Officials (AASHTO). A Policy on Geometric Design of Highways and Streets. 5th Edition, 2004.
13. Shinar, D. and R. Compton. Aggressive Driving: An Observational Study of Driver, Vehicle, and Situational Variables. *Accident Analysis and Prevention* 36, pp. 429-437, 2004.
14. Sharma, A., Bullock, D., Velipasalar, S., Casares, M., Schmitz, J., and Burnett, N. "Improving Safety and Mobility at High Speed Intersections with Innovations in Sensor Technology." 90th Transportation Research Board Annual Meeting, Transportation Research Board, Washington, DC, January 2011.
15. Sharma, A. Integrated Behavioral and Economic Framework for Improving Dilemma Zone Protection Systems. Ph.D. Dissertation. Purdue University, USA. 2008.
16. Glauber, C. Evaluation of a Dynamic Dilemma Zone Protection System at a Signalized Intersection. MS Thesis. Bradley University, USA. 2008.
17. Intersection of US 40 and Red Toad Road. Google. Google Maps. <http://maps.google.com>. Accessed June 16, 2012.
18. SmartSensor Advanced. Wavetronix. <http://www.wavetronix.com/en/products/smartsensor/advance>. Accessed June 16, 2012.

- 1 19. Long, K., Liu, Y., Han, D.L., “Impact of Countdown Timer on Driving Maneuvers
- 2 during Yellow Phase at Signalized Intersection: An Empirical Study in Changsha,
- 3 China,” Safety Science, in press.
- 4 20. Liu, Y., G. L. Chang, Yu, J., “Empirical study of driver responses during the yellow
- 5 signal phase at six Maryland intersections,” ASCE Journal of Transportation
- 6 Engineering, 138(1), pp. 31-42.
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