

EMPIRICAL OBSERVATIONS OF DRIVING BEHAVIORS AT INTERSECTIONS WITH RED LIGHT CAMERA DEPLOYMENT Sung Yoon Park¹, Chien-Lun Lan² and Gang-Len Chang³

Abstract

- \succ This study presents the results of a two-phase evaluation of the red-light cameras' (RLC) effects on traffic safety.
- Before-and-after study of RLC effectiveness
- The impact of RLC on the driving behaviors
- A properly deployed RLC program has the potential to
- Reduce side-impact crashes
- Decrease the percentage of aggressive drivers
- Encourage drivers to slow down and stop safely during the yellow phase
- Reduce red-light-running vehicles
- > Failing to inform drivers in advance of the RLC deployment may lead drivers to take improper decisions in the dilemma zone and result in rearend collisions.

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Types of be Type-1: reduct rear-end crash Type-2: reduct crashes but no

Type-3: reduct but not in side Type-4: no sign impact and rea

The incons

- Failures

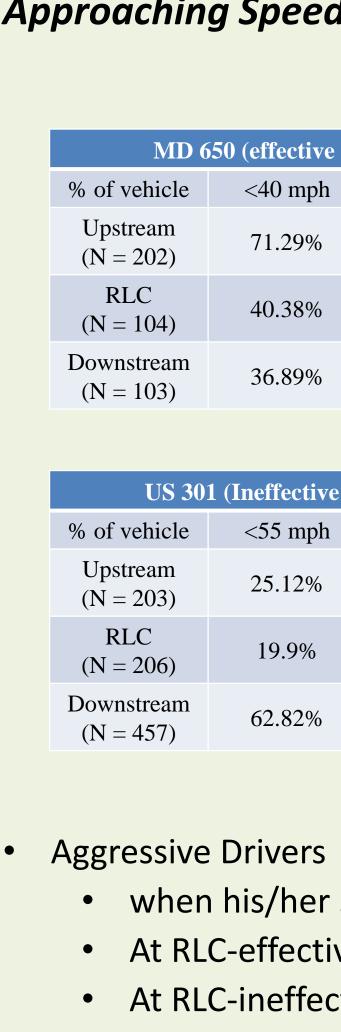
Design of Data Collection

	Upstream Intersection	RLC Intersection	Downstream Intersection 3
		PHOTO ENFORCED	
Data collection sites	Upstream	Red Light Camera	Downstream
Site 1: MD 650	Oakview Dr.	Adelphi Rd.	Northampton Dr.
Site 2: US 301	Frank Tippett Rd.	Rosaryville Rd. (Old Indian Head Rd.)	Fairhaven Ave.



Key traffic characteristics and behavioral data

- Speed evolution of an approaching vehicle
- Distance to the stop line onset of the yellow phase
- An individual driver's decision on taking either the "stop" or "pass" action
- Acceleration and deceleration rates of each approaching vehicle
- Number of vehicles crossing the intersection during allred and/or red phases
- Timestamp when a "passing" vehicle traverses the stop line



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C effective	Before-and-after Compa in M					
fore-and-after crash patterns	List of Literature [*]		Montgomery County		Howard Cou	ntv
ction in both side-impact and shes	[Government Report]: Brooksville, Clermont, Davie, Miami, Pinecrest, Council Bluffs, Davenport, Howard, Portland, Knoxville, Austin	M1 M2 M3	MD 355 @ Cheltenh MD 124 @ Gosher Shady Grove Rd. @ Rese	Rd. Harch Blvd.	1 US 40 @ N 2 US 1 @ C Prince George's	I. Ridge Rd. orridor Rd. County
ction only in side-impact not in rear-end crashes	[Literature]: Bochner et al. (2010), Erke et al. (2009), Høye et al. (2013), Kangwon et al.(2007), Ko et al. (2013), Persaud et al. (2005), Radali et al. (2001), Retting et al. (2002), Ahmed et al. (2015), Shin et al.(2007);	M4 M5 M6 M7 M8	MD 355 @ Middlebro MD 355 @ Halpine US 29 @ Fenton MD 355 @ Grosven MD 185 @ Knowles	Rd.P2St.P3or Ln.P4Ave.P3	2 MD 410 (3 US 301@ Old 1 4 MD 410 @ 5 US 301 @	ernor Bridge Rd. @ MD 450 ndian Head Rd. @ 64th Ave. McKendee
	[Government Report]: Phoenix, Scottsdale, San Diego, Apopka, Boynton Beach, Campbellton, Fort Lauderdale, Manatee, New Port Richey, Ocoee, Palatka, Palm Beach, Sarasota, West Park, Lafayette, Greensboro, Newark, Suffolk, Amarillo, Denton, Diboll, Frisco, Mesquite, Port Lavaca, Fairfax, Falls Church, Vienna	M9 M10 M11 M12 M13 M14	US 29 @ MD 193 MD97 @ US 29 US 29 @ Tech R MD 97 @ Nirbeck MD 355 @ Montgom MD 185 @ Randolp	d. P Rd. P ery Ln. P1	7 MD 410 WE 8 MD 223 @ C 9 MD 301 @ Pc	Adelphi Rd. 8 @ Ager Rd. Id Branch Rd. inter Ridge Dr. Aarlboro Pike
ction only in rear-end crashes le-impact crashes		M15	MD 650 @ Adelph Summary for Side-Impact	i Rd.)	
gnificant impacts in both side- ear-end crashes	[Literature]: Claros et al. (2017); [Government Report]: Boca Raton, Clewiston, Jacksonville, Lakeland, Maitland, Miami Beach, Miami Spring, Orange, Orlando, Osceola, Palm Coast, Sunrise, Tamarac, Tampa, West Miami,	Length of Before After B:5-yr A:3-yr	Side-Impact Side-Impact Injury PDO "Increase" "Increase" P3	Side-Impact Side-In Injury PDO "Decrease" "Decre H1 ^{AR} , H2 ^A , M1, M2 ^A , M6, M7, M8, M12, P P8 H1 ^R , H2, M1, M2A, M	mpact Side-Impact Side-Impact Injury PDO ease" "Increase" "Decrease" M5, M13 ^R , P10 P2, P4,	Side-ImpactSide-ImpactInjuryPDO"Decrease""Increase"M3, M4 ^A , M11 ^A , M14,M15, P1, P5, P6 ^R , P7, P9 ^R M4, M12, M14 ^A , M15, P2,
• • • • •	Bedford, Cleveland, Garland, Haltom City, Richland Hills, University Park, Willis, Arlington	B:3-yr A:3-yr B:2-yr A:3-yr B:5-yr A:2-yr	M3, M5, P3, P10	M7, M8, P1, P4 H1, H2 ^A , M2 ^A , M6, M M15, P1 ^R , P4 H1A, H2, M1, M2 ^A , M	18, M1, M7, M12, M13 ^R , P8	P5, P6, P7, P9,M11 ^A M4 ^R , M14 ^R ,M11 ^A ,P2, P5, P6, P7, P9 ^R M4 ^A , M10, M14, M15, P1,
nsistencies in evaluation es to account for regres	n findings are likely attributed to sion to mean	B:3-yr A:2-yr B:2-yr A:2-yr B:2-yr A:2-yr	M9, M12, P3	M5, M6, M7, M8, P4 H1, H2, M2 ^A , M5, M P1, P4 H1, H2 ^A , M2A, M6, M P1, P4 ^A	7, M8, M1, M3, M13 ^R , P8, P10	 P2, P5, P6, P7, P9,M11 M4, M6, M10, M14, M15, P2, P5, P6, P7, P9, M11^A M4^R, M9, M10, M14, M15, P2, P5, P6, P7, P9, M11^A
nce of spillover/halo eff	fects	PDO: Property Da	mage Only	laval		

Existence of spillover/halo effects

Empirical Observation Results

Approaching Speed Distributions

650 (effective in reducing side-impact crashes; speed limit: 40 MPH)							
<40 mph	40 - 45 mph	45-50 mph	>50 mph	Average			
71.29%	14.85%	12.87%	0.99%	35.3			
40.38%	36.54%	13.46%	9.62%	41.5			
36.89%	33.98%	21.36%	7.77%	41.9			

1 (Ineffective in reducing side-impact crashes; Speed limit: 55 MPH)						
<55 mph	55-60 mph	60 – 65 mph	>65 mph	Average		
25.12%	24.14%	30.54%	20.20%	59.1		
19.9%	16.02%	24.27%	39.81%	61.5		
62.82%	19.23%	11.54%	6.41%	54.7		

• when his/her speed is +10mph than speed limit • At RLC-effective intersection (MD 650): 9.62% • At RLC-ineffective intersection (US 301): 39.81% Spillover effect: reduced the percentage of aggressive drivers at the downstream intersection

Speed Change during Yellow Phase

Moderate "Passing" Drivers						
Site	Intersection	Difference bet	Difference between the passing speed (at the stop line)			
		and the approaching speeds (700ft)				
		< -5mph	Unchanged	> 5mph		
MD650	Upstream	46 %	43 %	11 %		
(Effective)	RLC	7 %	57 %	36 %		
	Downstream	13 %	75 %	12 %		
US310	Upstream	9 %	56 %	35 %		
(Ineffective)	RLC	8 %	46 %	46 %		
	Downstream	20 %	75 %	5 %		
	Ag	gressive "Passing	" Drivers			
Site	Intersection	Difference bet	Difference between the passing speed (at the stop line)			
		and the approaching speeds (700ft)				
		< -10 mph	Unchanged	> 5mph		
MD650	Upstream	6.7 %	20 %	10 %		
(Effective)	RLC	29 %	36 %	7 %		
	Downstream	30 %	60 %	0 %		
US310	Upstream	0 %	89 %	11 %		
(Ineffective)	RLC	12 %	41 %	35 %		
	Downstream	40 %	20 %	0 %		

- its downstream intersection
- its downstream intersection
- Spillover effect

Side-impact crashes significant at the 90% confidence leve R: Rear-End crashes significant at the 90% confidence leve AR: Both side-impact and rear-end crashes significant at the 90% confidence level

Moderate speed drivers, who decelerate when passing the intersection during yellow phase increase from the RLC to

• MD 650: $7\% \rightarrow 13\%$; US 301: $8\% \rightarrow 20\%$

Moderate speed drivers, who accelerate when passing the intersection during yellow phase **decrease** from the RLC to

• MD 650: $36\% \rightarrow 12\%$; US 301: $46\% \rightarrow 5\%$

Effects on driving behaviors in the Dilemma Zone

Site	Intersection	Choose to stop within their "must-go" zone (rear-end collisions)	Choose to pass within their "must-stop" zone (side-impact crash)	Vehicles trapped in DZ	Total No. of vehicles encountering the yellow phase	
MD 650	Upstream	0.4% (1)	5.9% (15)	23.7% (60)	253	
	RLC	12% (32)	0.7% (2)	6.7% (18)	267	
(Effective)	Downstream	6.1% (12)	2.3% (5)	5.1% (10)	196	
	Upstream	0.5% (2)	0.9% (4)	30.1% (131)	435	
US 301	RLC	3.9% (21)	1.3% (7)	37.4% (202)	540	
(Ineffective)	Downstream	2.4% (7)	4.7% (14)	27.0% (80)	296	
MD 450 (Effective)		10.11% (9)	1.12% (1)	13.48% (12)	89	
MD 97 (Ineffective)		2.94% (4)	0.74% (1)	29.41% (40)	136	
* Numbers in parenthesis are number of cases observed during field data collection						

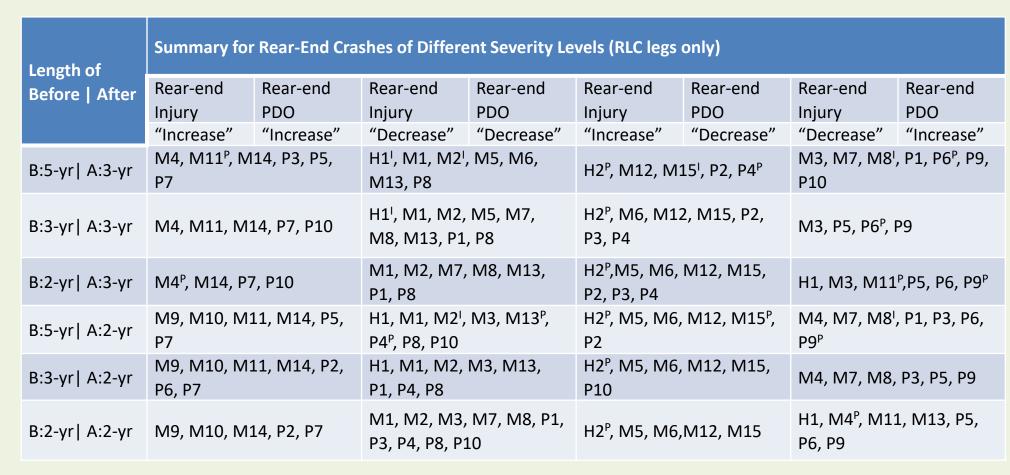
• The percentages of drivers who decided to stop when they were actually within the "must-go" zone

- MD 650: 12%; US301: 3.9%
- Those drivers might cause more rear-end collisions. Only a relatively small percentage of drivers were observed to pass when they were in "must-stop" zone.
- Such drivers are at risk of causing side-impact crashes.
- The percentage of drivers (37%) trapped in the dilemma zone at the RLC-ineffective intersection (US 301) was much higher than at RLC-effective intersection (MD 650, 6.7%)



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I: Injury crashes significant at the 90% confidence level P: PDO crashes significant at the 90% confidence level

IP: Both injury and PDO crashes significant at the 90% confidence level

Findings are consistent with the literature

- Reductions in side-impact crashes at most intersections with RLC
- The percentage of intersections with RLC had an increase of rear-end collisions and it was at approximately the same level as those reported to have positive effects
- A small percentage of RLC intersections seem to suffer from an increase in both rear-end and side-impact crashes

Conclusions

Findings from the Two-Phase Evaluations

- Proper implementation of the RLC program has reduced side-impact crashes, but not rear-end collisions
- RLC may either increase or decrease the number of rear-end collisions (depends on behavior of the driving populations)
- RLC reduced the percentage of aggressive drivers at both the RLC and its downstream intersection
- A properly implemented RLC program has significant influence on the behaviors of drivers
- A properly implemented RLC intersection was shown to have a spillover effect to neighboring intersections.

Future Study

• Due to the limited resources this study includes four intersections for data observations. Further analysis with different locations might be needed to generalize the effectiveness of the RLC